

FINITE ELEMENTS

HOMWORK 2

Plane Elasticity

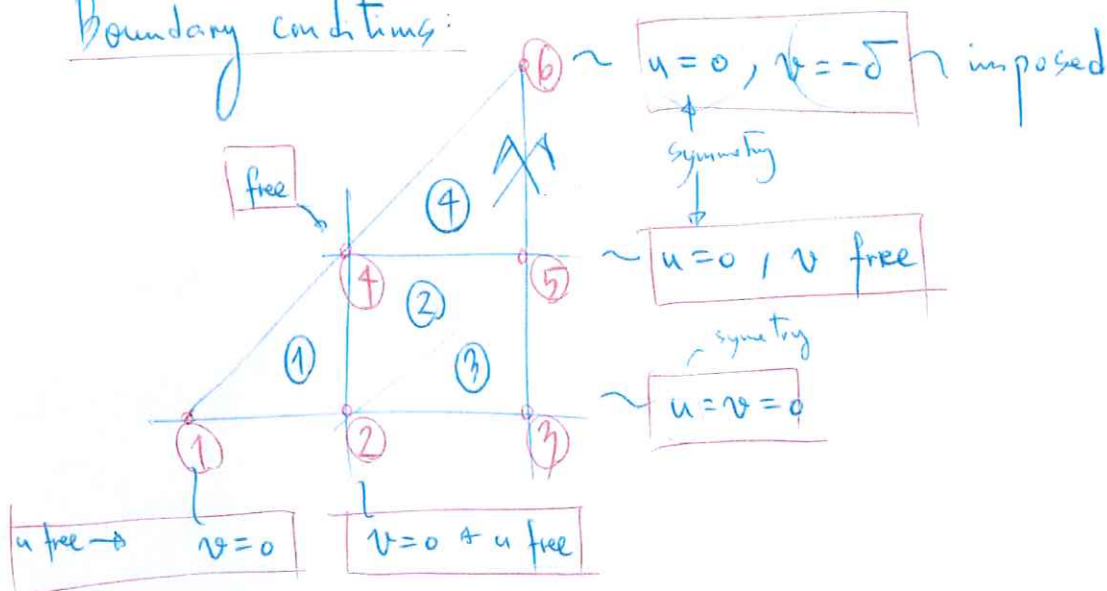
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1. Plane stress $\Rightarrow \boxed{\sigma_z = \tau_{xz} = \tau_{yz} = 0}$

$\underline{\sigma} = \underline{D} \cdot \underline{\epsilon}$, where $\begin{pmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{pmatrix} = \begin{pmatrix} d_{11} & d_{12} & 0 \\ d_{21} & d_{22} & 0 \\ 0 & 0 & d_{33} \end{pmatrix} \cdot \begin{pmatrix} \epsilon_x \\ \epsilon_y \\ \tau_{xy} \end{pmatrix}$

And $\boxed{d_{11} = d_{22} = \frac{E}{1 - \nu^2}}$; $\boxed{d_{12} = d_{21} = \frac{\nu \cdot E}{1 - \nu^2}}$; $\boxed{d_{33} = \frac{E}{2(1 + \nu)} = G}$

Boundary conditions:



2.

Node	Coordinates
1	(0, 0)
2	(1.5, 0)
3	(3, 0)
4	(1.5, 1.5)
5	(3, 1.5)
6	(3, 3)

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Element	Nodal connections		
1	2 ¹	4 ²	1 ³
2	4 ¹	2 ²	5 ³
3	3 ¹	5 ²	2 ³
4	5 ¹	6 ²	4 ³

local numbering

global numbering

2.

k_{33}^1	k_{13}^1	0	k_{23}^1	0	0
$k_{11}^1 + k_{22}^1 + k_{33}^1$	k_{13}^3	k_{11}^3	$k_{12}^1 + k_{12}^2$	$k_{23}^2 + k_{23}^3$	0
	0	0	k_{12}^3	0	0
	$k_{22}^1 + k_{11}^2 + k_{33}^3$	$k_{13}^2 + k_{14}^4 + k_{33}^3$	k_{23}^4		
		$k_{33}^2 + k_{33}^4 + k_{22}^3$	k_{12}^4		
			k_{22}^4		

SYMM.

 \bar{a}_1 \bar{a}_2 \bar{a}_3 \bar{a}_4 \bar{a}_5 \bar{a}_6

reactions at the bottom

 $\bar{r}_1 + f_3^1$ $r_2 + f_1^1 + f_2^2 + f_3^3$ $r_3 + f_1^3$ $f_2^1 + f_1^2 + f_3^4$ $f_3^2 + f_2^3 + f_1^4$ f_4

surface density should be added at nodes

Initially, the system has 12 unknowns. Nevertheless, the 7 boundary conditions leaves the problem with only 5 unknowns.

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4. We must set up the standard system of equations $\underline{K} \cdot \underline{a} = \underline{f}$

where $\underline{K}_e^{ij} = \int_A \underline{B}^T \underline{D} \underline{B} \cdot t \cdot dA$

\int_A (area of triangle) \underline{B} (deformation matrix) \underline{D} (constitutive matrix) t (thickness)

and $\underline{f}_e^i = \int_A \underline{N}_i^T \underline{b} \cdot t \cdot dA = \frac{At}{3} \begin{pmatrix} 0 \\ -\rho g \end{pmatrix}$

\underline{N}_i^T (thickness) \underline{b} (surface gravity) $-\rho g$ (gravity)

After calculating \underline{K} , we obtain the following structure:

$\underline{K} =$

$5 \cdot 10^9$	$-5 \cdot 10^9$		$-1 \cdot 10^9$		
1		0	$-4 \cdot 10^9$	$3 \cdot 10^9$	$-3 \cdot 10^9$
	$1.5 \cdot 10^{10}$				
		1			
		1			
			1		
0	0		$1.5 \cdot 10^{10}$	$-3 \cdot 10^9$	$3 \cdot 10^9$
			$1.5 \cdot 10^{10}$		$-1 \cdot 10^9$
				1	$-4 \cdot 10^9$
			$1.5 \cdot 10^{10}$		$5 \cdot 10^9$
				1	
					$5 \cdot 10^9$

$\underline{f} =$

0
-112.5
0
-138.9
0
-155

To properly calculate \underline{f} and reactions \underline{R} , we just follow the following methodology (skematic)

$$\left(\begin{array}{|c|} \hline \text{[Diagram of stiffness matrix K]} \\ \hline \end{array} \right) \left(\begin{array}{c} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{array} \right) - \left(\begin{array}{c} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{array} \right) = \left(\begin{array}{c} \text{[Diagram of reaction vector R]} \\ \hline \end{array} \right)$$

$\underline{K} \quad \underline{q} \quad \underline{f} \quad \underline{R}$

} reactions at the corresponding nodes

After fully solving the system, we obtain the solution \bar{u}^h :
In meters.

